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PROJECT INFORMATION:

CONSTRUCTION TYPE: _____ TYPE III-B
 OCCUPANCY TYPE: _____ GROUP A-3
 NUMBER OF STORES: _____ ONE w/ MEZZANINE
 STANDARD LOADINGS:
 SNOW LOAD: _____ 32 PSF
 ROOF DEAD LOAD: _____ PER METAL BLDG. MFR.
 ROOF LIVE LOAD: _____ 20 PSF
 FLOOR DEAD LOAD: _____ 20 PSF
 MEZZANINE FLOOR LIVE LOAD: _____ 100 PSF
 WIND LOAD: _____ 90 MPH, EXP. C
 SDC D; SITE CLASS D;
 SEISMIC DESIGN: _____ Ss=0.622; S1=0.214

BUILDING AREA/HEIGHT:
 MAIN FLOOR AREA: _____ 12750 SQ. FT.
 MEZZANINE FLOOR AREA: _____ 2918 SQ. FT.
 TOTAL AREA: _____ 15668 SQ. FT.
 BUILDING EAVE HEIGHT: _____ 22'-0"
 BUILDING OVERALL HEIGHT: _____ 26'-2 1/2"

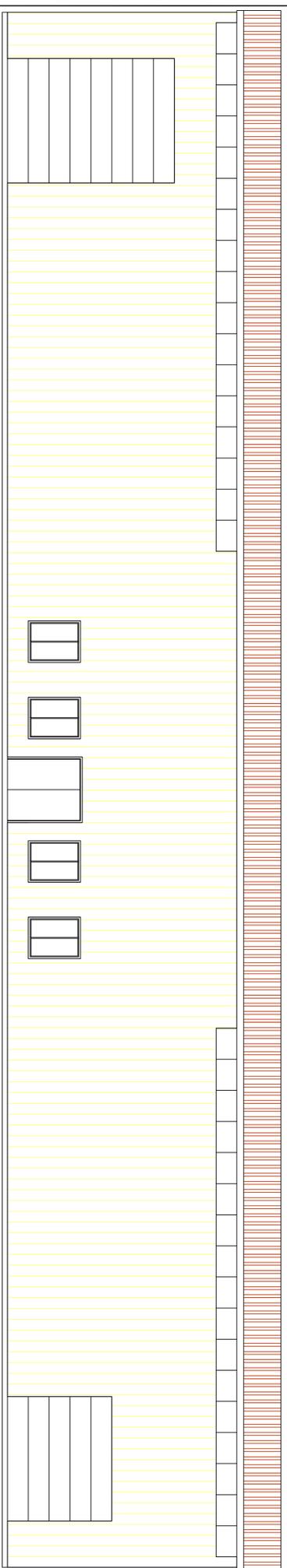
OCCUPANT LOAD (IBC TABLE 1004.1.1):
 MAIN FLOOR ASSEMBLY AREA: _____ 813
 (MULTI-PURPOSE ROOMS 4483 SQ. FT. EA. SIDE
 (TOTAL OF 8966 SQ. FT.) / 15 SQ. FT. / OCC +
 CONFERENCE ROOMS 1501 SQ. FT. / 7 SQ. FT. /
 OCC + STORAGE AREA 430 SQ. FT. / 300 / OCC +
 KITCHEN AREA 276 SQ. FT. / 200 / OCC)
 MAIN FLOOR LOBBY AREA: _____ 37
 (ASSEMBLY AREA 552 SQ. FT. / 15 / OCC)
 MAIN FLOOR STORAGE AREA: _____ 2
 (STORAGE AREA 430 SQ. FT. / 300 / OCC)
 MAIN FLOOR KITCHEN AREA: _____ 2
 (KITCHEN AREA 276 SQ. FT. / 200 / OCC)
 MEZZANINE STORAGE AREA: _____ 10
 (STORAGE AREA 2912 SQ. FT. / 300 SQ. FT. / OCC)
 TOTAL OCCUPANT LOAD: _____ 864

CODE REQUIREMENTS:
 *BUILDING CONSTRUCTED AS MIXED USE, NON-SEPARATED OCCUPANCY
 *3 SEPARATE FIRE AREAS SEPARATED BY 2-HR FIRE WALLS

DEFERRED SUBMITTALS:
 - METAL BUILDING PLANS AND CALCULATIONS
 - SHEET ROCK INSTALLATION DETAIL
 - LIGHTING INSTALLATION DETAILS

SPECIAL INSPECTION REQUIREMENTS:
 SPECIAL INSPECTION OF ALL EPOXY APPLICATIONS REQ'D

ALL CONSTRUCTION SHALL COMPLY TO THE FOLLOWING CODES:
 2009 INTERNATIONAL PLUMBING CODE
 2009 INTERNATIONAL MECHANICAL CODE
 2009 INTERNATIONAL BUILDING CODE
 2009 INTERNATIONAL FIRE CODE
 2009 INTERNATIONAL ENERGY CONSERVATION CODE
 2008 NATIONAL ELECTRIC CODE
 2003 ANSI 117.1



DATE: MAY. 2011	FILE: KC
SHEET: T	DRAWN BY: RMD

TC ENGINEERING, PC
 A "DESIGN-BUILD" FIRM
 EXCELLENCE...ON TIME!
 DANIEL W. THEBEAU, P.E.
 P.O. BOX 55, KANAB, UTAH 84741
 (435)644-2031 (888)644-2031
 (435)689-0155

KC FAIRGROUNDS BUILDING
TITLE SHEET
KANE COUNTY, UTAH
 SCALE= NTS
 REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

DATE: 5/16/11	REVISION: INITIAL DESIGN
	OWNER REVIEW
	CITY APPROVAL
	FOR CONSTRUCTION

WINDOW SCHEDULE			
MARK	WIDTH	HEIGHT	Style Header Ht. Remarks
11	4'-0"	5'-0"	Slider 7'-0"
12	4'-0"	5'-0"	Slider 7'-0"
13	4'-0"	5'-0"	Slider 7'-0"
14	4'-0"	5'-0"	Slider 7'-0"
15	4'-0"	5'-0"	Slider 7'-0"
16	4'-0"	5'-0"	Slider 7'-0"
17	4'-0"	5'-0"	Slider 7'-0"
18	4'-0"	5'-0"	Slider 7'-0"

DOOR AND FRAME SCHEDULE

MARK	Style	DOOR SIZE			Swing	FR	Notes	WALL SIZE
		WD	HGT	THK				
101	Hinged - Double	PR 3'-0"	7'-0"	1 3/4"	Right	--	--	8"
102	Overhead - Garage Door	12'-0"	16'-0"	1 3/4"	Right	--	ROLLUP - AUTO	8"
103	Hinged - Double - Exterior	PR 3'-0"	7'-0"	1 3/4"	Right	--	--	8"
104	Overhead - Garage Door	12'-0"	10'-0"	1 3/4"	Right	--	ROLLUP - AUTO	8"
105	Hinged - Double - Exterior	PR 3'-0"	7'-0"	1 3/4"	Right	--	--	8"
106	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	1 1/2 HR	SELF CLOSING	5 1/2"
107	Hinged - Double	PR 3'-0"	6'-8"	1 3/8"	Left	1 1/2 HR	SELF CLOSING	5 1/2"
108	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	1 1/2 HR	SELF CLOSING	5 1/2"
109	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	1 1/2 HR	SELF CLOSING	5 1/2"
110	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	1 1/2 HR	SELF CLOSING	5 1/2"
111	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	1 1/2 HR	SELF CLOSING	5 1/2"
112	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	1 1/2 HR	SELF CLOSING	5 1/2"
113	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	1 1/2 HR	SELF CLOSING	5 1/2"
114	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	1 1/2 HR	SELF CLOSING	5 1/2"
115	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	--	--	5 1/2"
116	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	--	--	5 1/2"
117	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	--	--	5 1/2"
118	Hinged - Double	PR 2'-10"	6'-8"	1 3/8"	Right	--	--	5 1/2"
119	Hinged - Double	PR 2'-10"	6'-8"	1 3/8"	Left	--	--	5 1/2"
120	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	--	--	5 1/2"
121	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	--	--	5 1/2"
122	Hinged - Single	3'-0"	6'-8"	1 3/8"	Right	--	--	5 1/2"

WINDOW SCHEDULE			
MARK	WIDTH	HEIGHT	Style Header Ht. Remarks
21	4'-0"	5'-0"	Slider 7'-0"
22	4'-0"	5'-0"	Slider 7'-0"
23	4'-0"	5'-0"	Slider 7'-0"
24	4'-0"	5'-0"	Slider 7'-0"

DOOR AND FRAME SCHEDULE

MARK	Style	DOOR SIZE			Swing	FR	Notes	WALL SIZE
		WD	HGT	THK				
201	Hinged - Single	3'-0"	6'-8"	1 3/8"	Left	--	--	5 1/2"

SC1 MAIN FLOOR DOOR/WINDOW SCHEDULES
SCALE: NTS

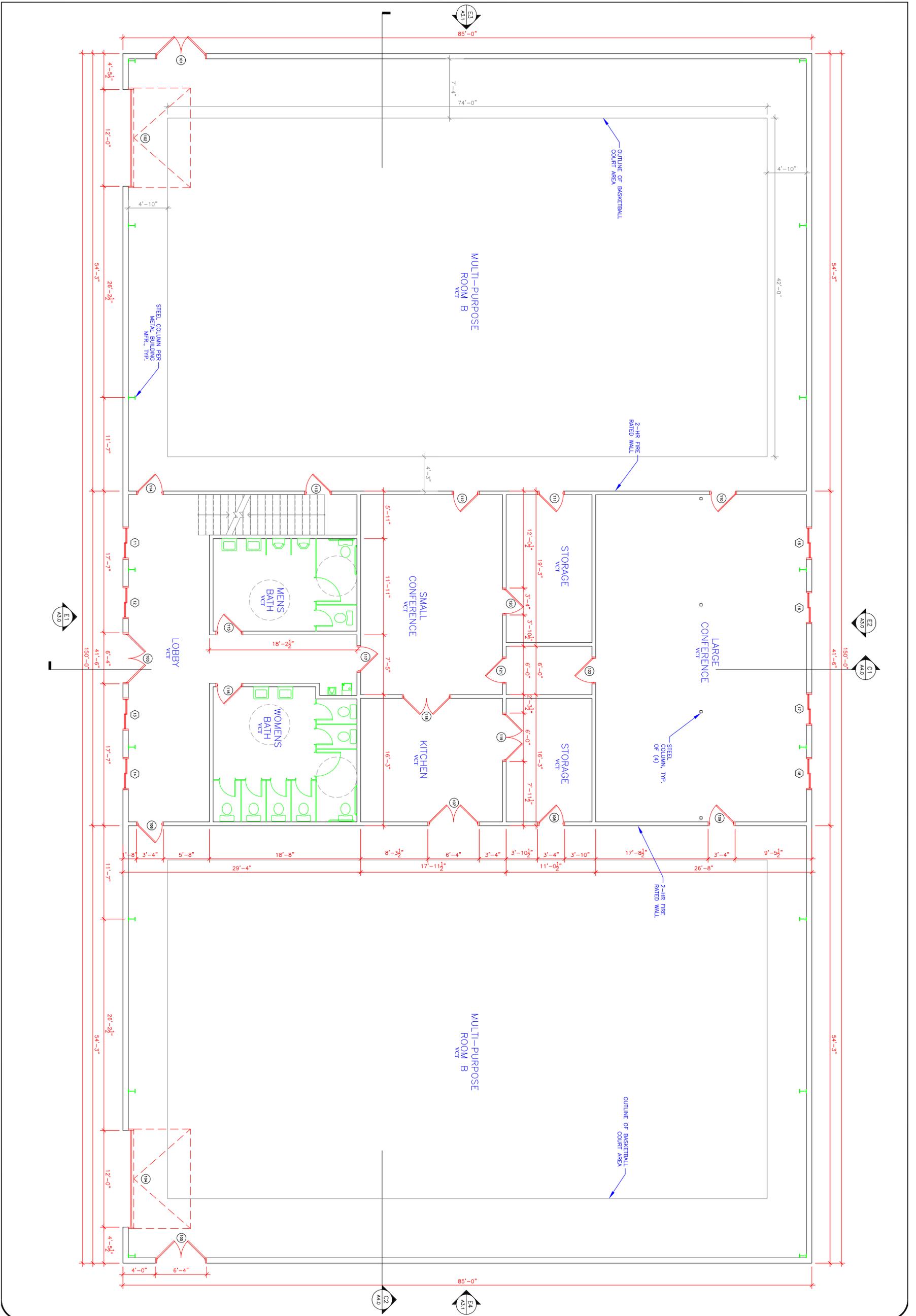
SC2 UPPER FLOOR DOOR/WINDOW SCHEDULES
SCALE: NTS

DATE:	REVISION:
5/16/11	INITIAL DESIGN
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	FOR CONSTRUCTION

KC FAIRGROUNDS BUILDING
DOOR/WINDOW SCHEDULES
KANE COUNTY, UTAH
SCALE= NTS
REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

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DATE:	FILE:
MAY. 2011	KC
SHEET:	DRAWN BY:
A0.1	RMD

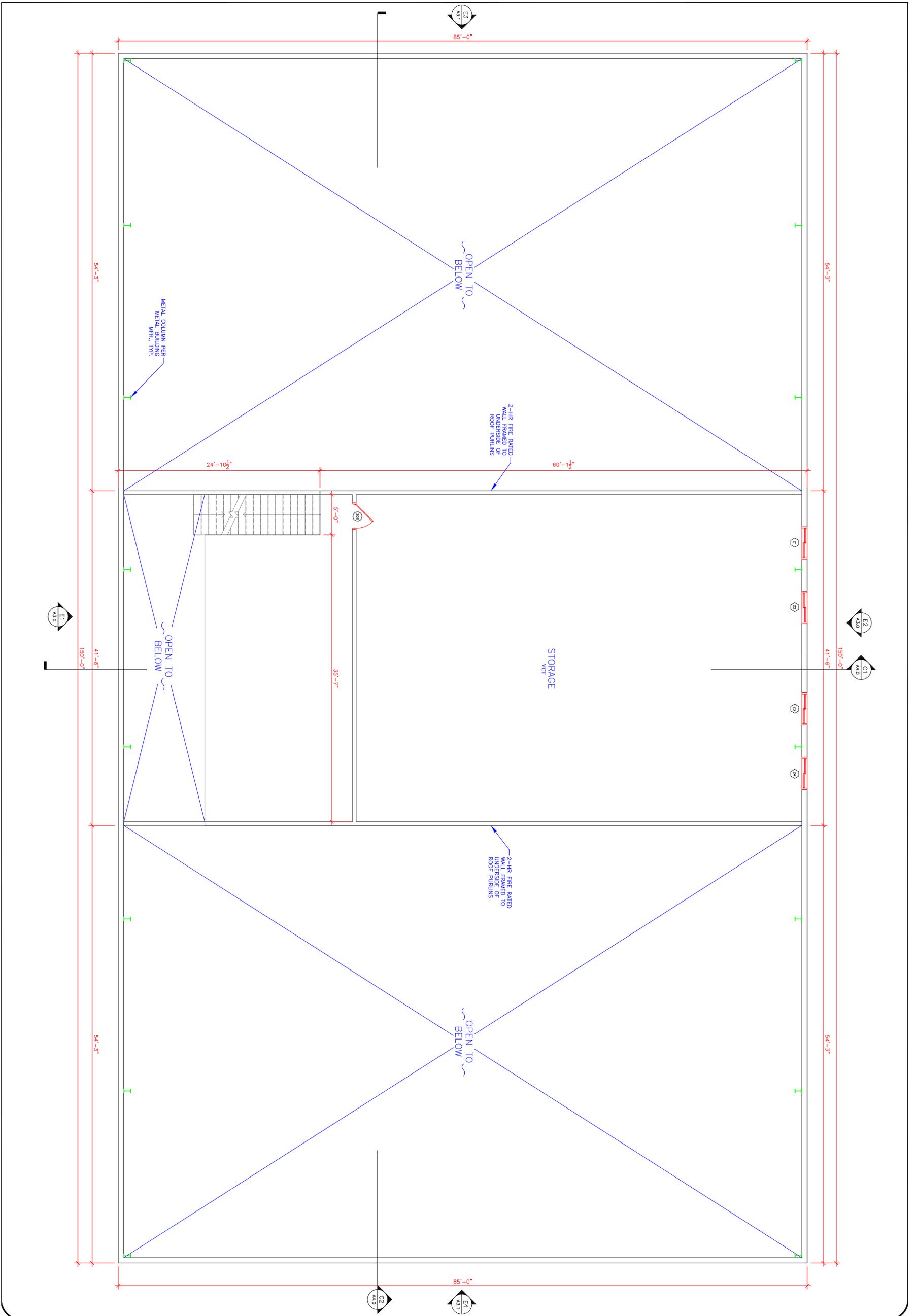


DATE: MAY. 2011	FILE: KC
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KC FAIRGROUNDS BUILDING
MAIN FLOOR PLAN
KANE COUNTY, UTAH
 SCALE= 3/16"=1'-0"
 REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

DATE: 5/16/11	REVISION: INITIAL DESIGN
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	FOR CONSTRUCTION



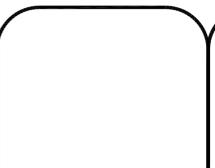
DATE:
MAY. 2011

SHEET:
A2.0

FILE:
KC

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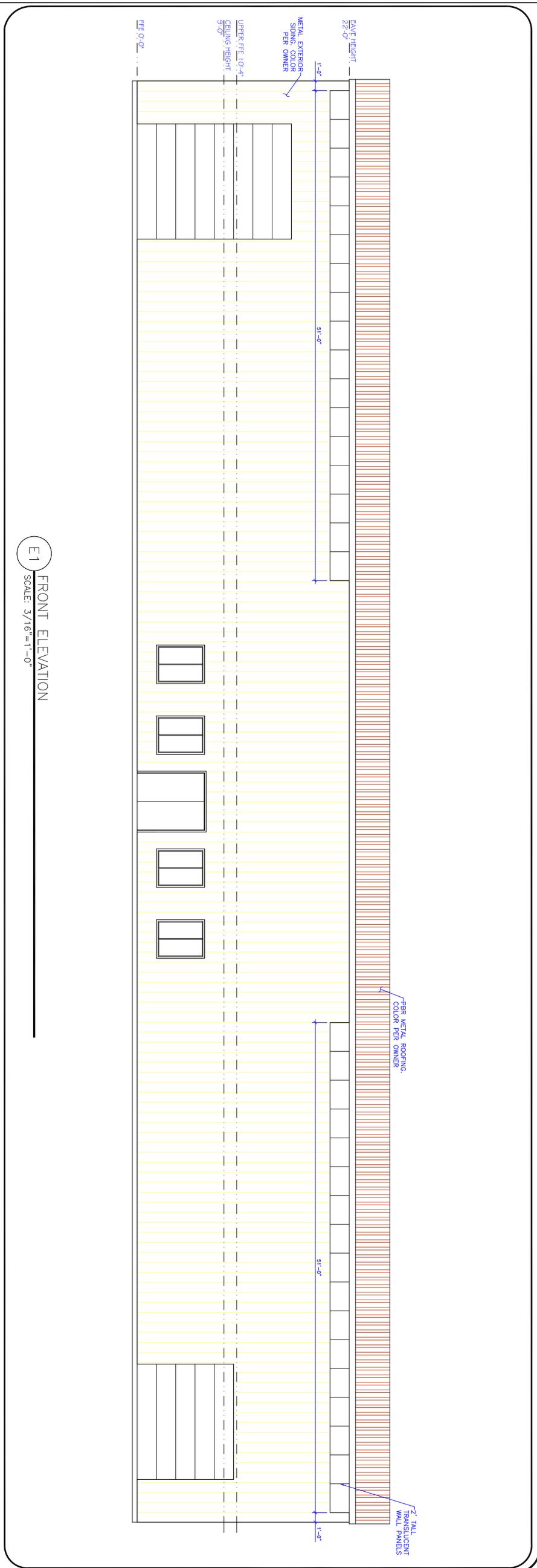
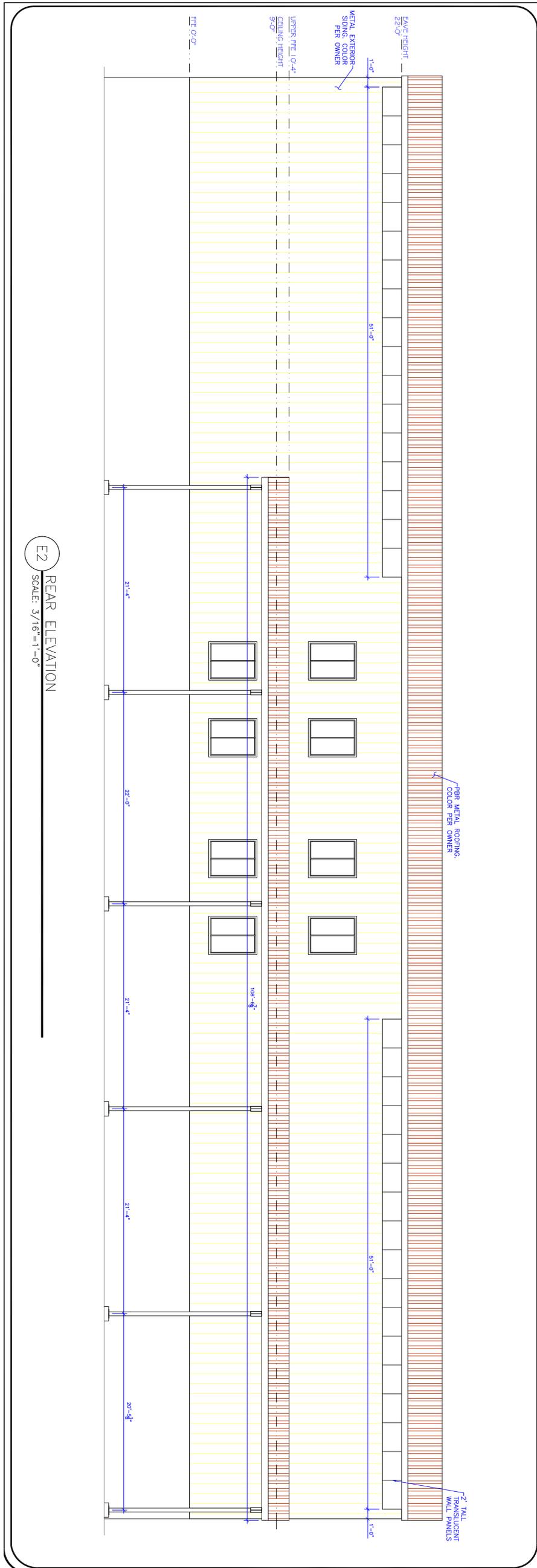
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KC FAIRGROUNDS BUILDING
UPPER FLOOR PLAN
KANE COUNTY, UTAH

SCALE= 3/16"=1'-0"
 REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

DATE:	REVISION:
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	FOR CONSTRUCTION



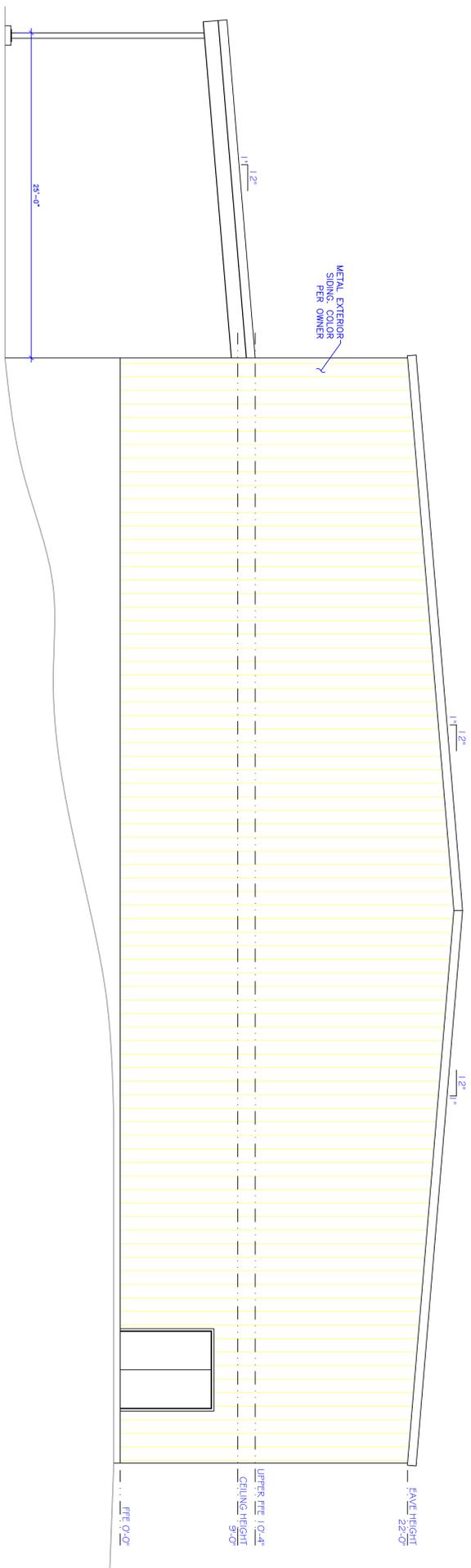
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 SHEET: A3.0

FILE: KC
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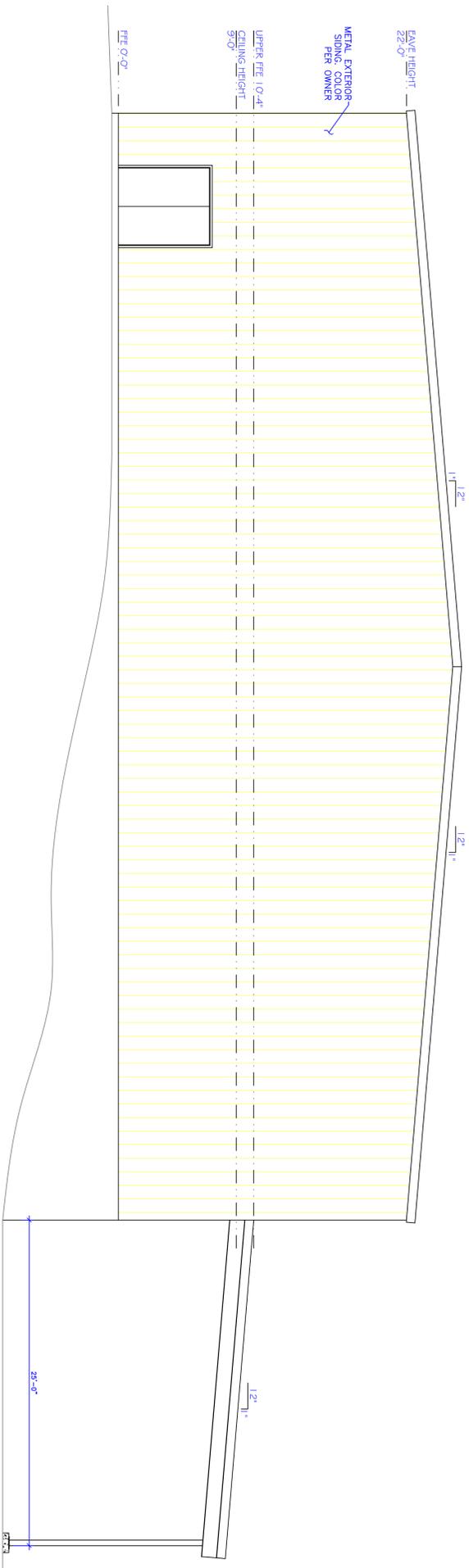
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**KC FAIRGROUNDS BUILDING
 ELEVATIONS
 KANE COUNTY, UTAH**
 SCALE= AS NOTED
 REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

DATE:	REVISION:
5/16/11	INITIAL DESIGN
	OWNER REVIEW
	CITY APPROVAL
	FOR CONSTRUCTION



E3 LEFT ELEVATION
SCALE: 3/16"=1'-0"



E4 RIGHT ELEVATION
SCALE: 3/16"=1'-0"

DATE: MAY. 2011	FILE: KC
SHEET: A3.1	DRAWN BY: RMD

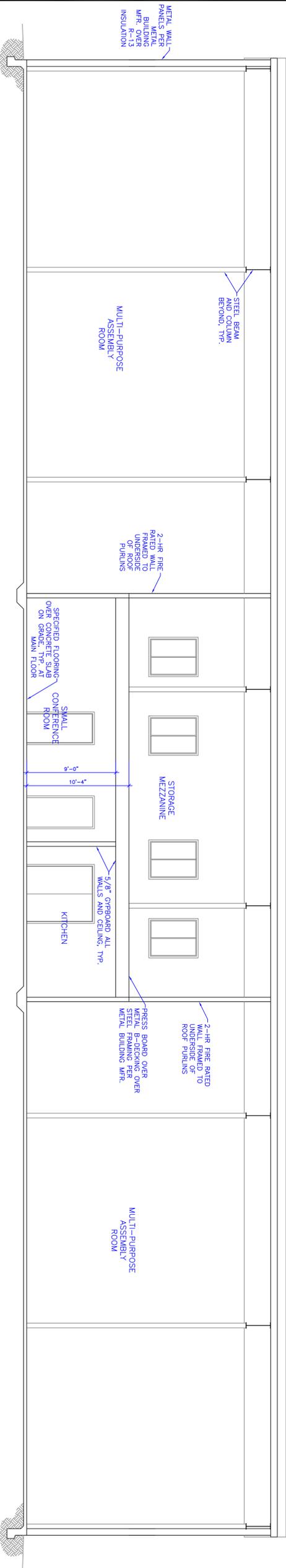
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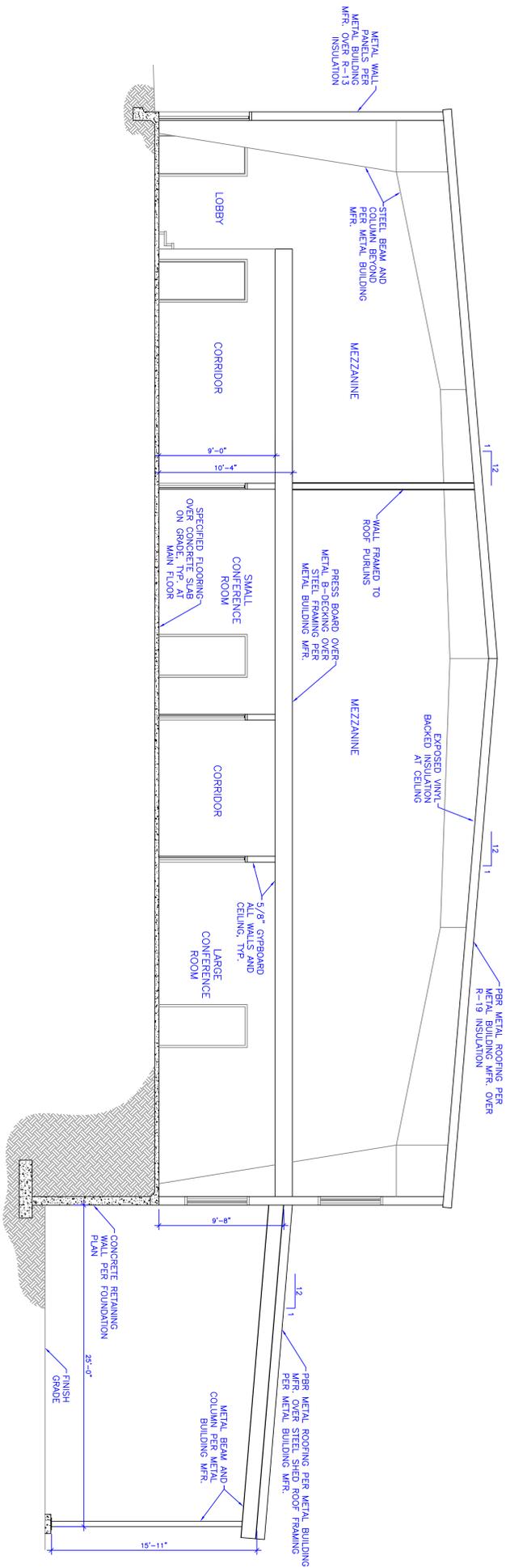
**KC FAIRGROUNDS BUILDING
ELEVATIONS
KANE COUNTY, UTAH**

SCALE= AS NOTED
REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

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C2 CROSS SECTION
SCALE: 3/16"=1'-0"



C1 CROSS SECTION
SCALE: 3/16"=1'-0"

DATE:	REVISION:
5/16/11	INITIAL DESIGN
	OWNER REVIEW
	CITY APPROVAL
	FOR CONSTRUCTION

KC FAIRGROUNDS BUILDING
CROSS SECTIONS
KANE COUNTY, UTAH
SCALE= AS NOTED
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DATE: MAY. 2011	FILE: KC
SHEET: A4.0	DRAWN BY: RMD

GENERAL NOTES

- DESIGN SNOW LOADS:
ROOF = 32 PSF
IMPORTANCE FACTOR = 1.0
- BASES FOR WIND DESIGN:
2009 INTERNATIONAL BUILDING CODE / ACE 7-05
WIND = 90 MPH BASIC WIND SPEED, EXPOSURE C.
EXPOSURE CATEGORY - C
WIND USE GROUP - II
IMPORTANCE FACTOR = 1.0
- SEISMIC - SITE CLASS D
SEISMIC DESIGN CATEGORY D
S_s = 0.222
S₁ = 0.214
- LATERAL FORCE RESISTING SYSTEM:
PRE-FABRICATED STEEL MOMENT/BRACED FRAME
- THESE STRUCTURAL NOTES DO NOT SUPERSEDE THE PLAN NOTES. CONSULT THE PLAN NOTES SPECIFIC TO FOUNDATION AND FRAMING FOR ADDITIONAL REQUIREMENTS IN EACH SECTION. IF CONFLICT BETWEEN NOTES AND SPECIFICATIONS OCCURS, THE MOST STRINGENT REQUIREMENT GOVERNS. NOTES AND DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES, TYPICAL DETAILS, AND SPECIFICATIONS.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. DURING CONSTRUCTION, THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO FABRICATION OR CONSTRUCTION IN ANY AREA. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES, OMISSIONS, OR INCONSISTENCIES. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENTS AS DIRECTED BY THE ARCHITECT AND ENGINEER WITHOUT ADDITIONAL COST TO THE OWNER. DO NOT SCALE DRAWINGS!
- ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE INTERNATIONAL BUILDING CODE, ANY OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, AND THE CODES AND STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS. ALL SPECIFICATIONS NOTED SHALL BE THE LATEST APPROVED REVISION OF THE CODES AND STANDARDS. THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTING THEM TO THE ARCHITECT. A REVISIONED COPY OF ALL SHOP DRAWINGS SHALL BE KEPT AT THE CONSTRUCTION SITE FOR REFERENCE. THE SHOP DRAWING REVIEW SHALL NOT RELIEVE THE GENERAL CONTRACTOR OF ANY RESPONSIBILITY FOR COMPLETION OF THE PROJECT ACCORDING TO THE CONTRACT DOCUMENTS.
- THE CONTRACTOR SHALL INVESTIGATE THE SITE DURING CLEARING, EXCAVATION OR OTHER EARTH WORK OPERATIONS FOR FILLED EXCAVATIONS, BURIED STRUCTURES OR UNNATURAL SOIL CONDITIONS.
- STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. NOT THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. THESE MEASURES SHALL BE APPROVED BY THE ARCHITECT PRIOR TO CONSTRUCTION. SHORING AND BRACING SHALL REMAIN IN PLACE UNTIL ALL PERMANENT MEMBERS ARE IN PLACE AND CONNECTIONS COMPLETE. OBSERVATION VISITS TO THE SITE BY THE ENGINEER OR HIS REPRESENTATIVE SHALL NOT INCLUDE INSPECTION OF THESE ITEMS. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOF. LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
- THE STRUCTURAL DRAWINGS ARE A PORTION OF THE COMPLETE SET OF CONSTRUCTION DOCUMENTS AND ARE NOT INTENDED TO CONVEY ABSOLUTELY ALL INFORMATION RELATED TO THE PRIMARY STRUCTURE OF THE GENERAL CONTRACTOR TO DOCUMENT. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO COORDINATE WITH ALL TRADES, ANY AND ALL ITEMS THAT ARE TO BE INTEGRATED INTO THE STRUCTURAL SYSTEM.
- SEE ARCHT. DRAWINGS FOR THE FOLLOWING: (UNLESS NOTED SPECIFICALLY ON STRUCTURAL DRAWINGS)
- SIZE AND LOCATION OF DOOR, WINDOW, FLOOR AND ROOF OPENINGS
- SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS
- FLOOR AND ROOF FINISHES
- STAR FRAMING AND DETAILS (EXCEPT AS SHOWN).
- DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR THE FOLLOWING: (UNLESS SHOWN OR NOTED)
- PIPE RUNS, SLEEVES, TRENCHES, HANGERS, WALL AND SLAB OPENINGS, ETC.
- ELECTRICAL CONDUITS, BOXES, AND OUTLETS IN WALLS AND SLABS.
- CONCRETE INSERT REQUIREMENTS FOR MECHANICAL AND ELECTRICAL.
- SIZE AND LOCATION OF MACHINE OR EQUIP. BASES, ANCHOR BOLT REQUIREMENTS, ETC.
- OPENINGS LARGER THAN 6 IN. SHALL NOT BE PLACED IN SLABS, DECKS, WALLS, ETC., UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW ABOVE CONDITIONS LOCATED IN STRUCTURAL MEMBERS.
- OBSERVATION VISITS BY THE ENGINEER OR HIS REPRESENTATIVE SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION.

FOUNDATIONS

- FOUNDATION AND FOOTINGS ARE DESIGNED BASED ON A BEARING PRESSURE OF 1500 PSF AS PER 2009 IBC RECOMMENDATIONS
- THE CONTRACTOR SHALL PROVIDE FOR PROPER DE-WATERING OF ANY AND ALL EXCAVATIONS IF REQUIRED.
- THE CONTRACTOR SHALL PROVIDE FOR THE DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, AND SHORING REQUIRED TO SAFELY AND ADEQUATELY RETAIN ANY EXCAVATIONS.
- ALL RETAINING WALLS, BUILDING WALLS, PITS, ETC. MUST HAVE ATTAINED THEIR DESIGN STRENGTH AND/OR SUPPORT PRIOR TO BACKFILLING. EXCEPTION - IF BRACING IS TO BE USED TO SUPPORT WALLS AND ETC. FOR EARLY BACKFILLING, CONTRACTOR IS RESPONSIBLE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING.
- GRADING SHALL ALLOW FOR POSITIVE DRAINAGE (2 PERCENT MINIMUM) AWAY FROM THE BUILDING. OTHER FOOTINGS AND FOUNDATIONS, DRAINS AND SIDEWALKS ALL DOWN SPOURS SHALL DRAIN ONTO 3 FOOT LONG SPLASH BLOCKS SLOPING AWAY FROM FOUNDATIONS.
- EXCESSIVE WETTING OR DRYING OF THE FOUNDATION EXCAVATION AND THE FLOOR SLAB AREAS SHOULD BE AVOIDED DURING CONSTRUCTION.
- ALL FILL SUPPORTING CONCRETE SLABS, FOOTINGS, OR ETC. SHALL BE MOISTENED AND COMPACTED TO AT LEAST 95 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557 (MODIFIED PROCTOR). ALL OTHER FILL SHALL BE COMPACTED TO A MINIMUM RELATIVE COMPACTION OF NINETY (90) PERCENT OF MAXIMUM DRY DENSITY. COMPACTION TESTING SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY AND THE RESULTS SUBMITTED TO THE STRUCTURAL ENGINEER. SUPERFICIAL FIELD DENSITY TESTS SHALL BE PERFORMED TO CERTIFY BUILDING PADS ARE CONFORMING TO THE SPECIFICATIONS.
- FOOTINGS SHALL BE PLACED ON A MINIMUM OF 18" OF STRUCTURAL FILL COMPACTED TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557.

WOOD CONSTRUCTION

- ALL PHASES OF WORK PERTAINING TO WOOD FRAMING OR WOOD CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS LISTED IN CHAPTER 23 OF THE IBC.
- ALL WOOD BEAMS, JOISTS AND COLUMNS SHALL BE #2 DOUGLAS FIR (D.F.) GRADE LUMBER OR BETTER (UNO.) MICRO-LAM BEAMS SHALL HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 2,800 psi.
- ALL GLUE LAMINATED TIMBER MEMBERS SHALL HAVE THE FOLLOWING MINIMUM STRESS GRADE LUMBER:
1. BENDING = 2400 psi
2. TENSION = 1700 psi
3. COMPRESSION PARALLEL TO GRAIN = 1650 psi
- GLUE LAMINATED STRUCTURAL MEMBERS SHALL CONFORM TO THE U.S. DEPARTMENT OF COMMERCE COMMERCIAL STANDARDS PS-96 AND SECTION 2312, TABLES 23-1-C AND 23-1-D OF THE IBC.
- ALL STRUCTURAL PLYWOOD SHALL BE STRUCTURAL I OR STRUCTURAL II GRADE. A.P.A. PERFORMANCE RATED WAFFERBOARD, COMPOSITE BOARD, AND ORIENTED STRAND BOARD (BUT NOT STRUCTURAL PARTICLE BOARD) ARE ACCEPTED AS EQUIVALENT TO PLYWOOD, PROVIDING SPECIFIED SPAN RATINGS AND OTHER SPECIFIED REQUIREMENTS FOR PLYWOOD ARE MET.
- ALL PLATES OR OTHER LUMBER IN CONTACT WITH CONCRETE OR WITHIN 6 INCHES OF EARTH SHALL BE FOUNDATION REDWOOD ALL MARKED OR BRANDED BY THE REDWOOD INSPECTION SERVICE OR PRESSURE TREATED FOR MOISTURE PROTECTION.
- PROVIDE APPROVED BRIDGING AT A MAXIMUM OF 8 FEET O.C. BETWEEN FLOOR JOIST SUPPORTS FOR ALL SPANS OVER 14 FEET.
- TRUSSES AND/OR WEB JOISTS SHALL HAVE ALL BLOCKING, BRACING, BRIDGING, AND ETC. AS RECOMMENDED BY THE MANUFACTURER.
- WALLS SHALL RUN CONTINUOUS BETWEEN HORIZONTAL SUPPORT POINTS, UNLESS ADEQUATE APPROVED BRACING IS PROVIDED.
- REQUIRED MINIMUM NAILING SCHEDULE FOR USE WHERE NOT NOTED OTHERWISE ON PLANS OR DETAILS: (SEE IBC TABLE NO. 23-0)
TOE NAIL 4-8d OR END NAIL 2-16d
DOUBLE TOP PLATES: _____ FACE NAIL 16 O.C. STAGGERED 1-16d w/ 2-16d AT LAPS AND INTERSECTIONS.
DOUBLE STUDS _____ FACE NAIL 24" O.C. 16d
CORNER STUD AND ANGLES _____ 24 O.C. 16d
JOIST TO SILL OR GIRDERS _____ TOE NAIL 3-8d OR 2-16d
SOLE PLATE TO JOIST/BLOCKING _____ FACE NAIL 16 O.C. 16d
BRIDGING TO JOIST _____ OR NAIL EACH END 2- 8d
PLYWOOD TO ROOF JOISTS, TRUSSES OR STUDS - SEE NAILING SCHEDULE.
- NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN FLUSH BUT SHALL NOT BREAK THE SURFACE OF THE SHEATHING.
- CONNECT ALL WOOD TO CONCRETE, WOOD TO STEEL, AND WOOD TO WOOD (EXCEPT STUD TO PLATE) WITH SIMPSON OR EQUAL CONNECTORS UNO.

CONCRETE

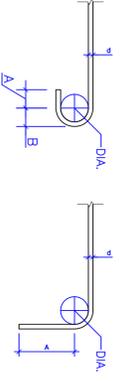
- ALL PHASES OF WORK PERTAINING TO THE CONCRETE CONSTRUCTION SHALL CONFORM TO THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318) AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 318) LATEST APPROVED EDITIONS, WITH MODIFICATIONS AS NOTED IN THE DRAWINGS OR SPECIFICATIONS.
- CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY. ALL CONCRETE IN CONTACT WITH THE EARTH SHALL CONTAIN FIVE (5) PORTLAND CEMENT UNLESS NOTED OTHERWISE (UNO). ALL CONCRETE SHALL BE AIR ENTRAINED BY 5% +/- 1%.
- CALCIUM CHLORIDE SHALL NOT BE USED.
- CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS WITHIN 28 DAYS AFTER PLACEMENT (UNO):
FOUNDINGS 4,000 psi
FOUNDATION 4,000 psi
INTERIOR FLATWORK 3,000 psi
ALL EXTERIOR CONCRETE 3,000 psi
- MAXIMUM CONCRETE SLUMP SHALL NOT EXCEED 4 INCHES.
- ALL CONCRETE SHALL BE THOROUGHLY CURED ACCORDING TO ACI RECOMMENDATIONS. FOLLOW ACI 306R "COLD WEATHER CONCRETING" AND ACI 305R "HOT WEATHER CONCRETING" FOR ALL CONCRETE AND MASONRY WORK WHEN REQUIRED BY CURRENT WEATHER CONDITIONS.
- CONDUITS AND PIPES EMBEDDED IN CONCRETE SHALL CONFORM TO THE REQUIREMENTS IN SECTION 1906.3 OF 2003 INTERNATIONAL BUILDING CODE.
- NO ALUMINUM OR ANY METAL INJURIOUS TO CONCRETE SHALL BE EMBEDDED IN CONCRETE.
- BOTH INTERIOR AND EXTERIOR CONCRETE SLABS-ON-GRADE SHALL BE A MINIMUM OF 4 INCHES IN THICKNESS UNO. WITH SPAIN OR TOILED JOINTS A MAXIMUM 12 FEET IN EACH DIRECTION. SPAIN JOINTS SHALL BE AT SLAB THICKNESS DEPTH AND SHALL BE IDENTIFIED AS SUCH ON DRAWINGS. JOINTS SHALL BE MADE AND LOCATED AS TO LEAST REPAIR ALL REINFORCING AND BARS SHALL BE CONTINUOUS THROUGH JOINTS (UNO).
10. CLEAR COVERAGE OF CONCRETE OVER OUTER REINFORCEMENT BARS SHALL BE AS FOLLOWS (UNO):
- FOR CONCRETE PLACED DIRECTLY AGAINST EARTH, 3 IN. COVER
- FOR CONCRETE SURFACES EXPOSED TO WEATHER, 1 1/2 IN. COVER
- FOR CONCRETE SURFACES EXPOSED TO GROUND AFTER REMOVAL OF FORMS, 2" COVER
- FOR CONCRETE SURFACES NOT EXPOSED TO THE GROUND OR WEATHER, SLABS AND WALLS 3/4 IN. COVER; JOISTS OR WAFFLE BEAMS, 1 IN. COVER; BEAMS, PIERS AND COLUMNS, 1 1/2 IN. COVER.
- WHERE CONCRETE GIRTHS, BEAMS, OR WALLS ARE CONTINUOUS AROUND A CORNER, ADD CORNER BARS TO LAP 40 BAR DIAMETERS IN EACH DIRECTION, REINFORCING BARS IN THE INTERIOR FACES SHALL EXTEND TO WITHIN 2 IN. OF THE OUTER FACE AND SHALL TERMINATE IN A STANDARD HOOK OR BEND.
- AROUND OPENINGS IN CONCRETE SLABS, UNLESS OTHERWISE SCHEDULED, ADD REINFORCING EQUIVALENT TO BARS CUT BY OPENING. THE BARS PARALLEL TO THE MAIN REINFORCEMENT SHALL RUN THE FULL LENGTH OF THE SPAN. THE BARS PARALLEL TO THE TEMPERATURE STEEL SHALL RUN 40 BAR DIAMETERS EACH WAY BEYOND THE OPENING.

REINFORCING STEEL (FOR CONCRETE AND MASONRY)

- ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318 LATEST EDITION) AND THE MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION (1973 EDITION) BY THE CRSI AND THE WCSRI, AS MODIFIED BY THE PROJECT DRAWINGS AND SPECIFICATIONS.
- CHAIRS, SUPPORTS AND THE BARS REQUIRED IN ADDITION TO THE SCHEDULED REINFORCING SHALL BE FURNISHED BY THE CONTRACTOR.
- ALL STEEL REINFORCEMENT SHALL CONFORM TO ASTM A615 GRADE 60 WITH A MINIMUM YIELD STRENGTH OF 60,000 psi, WITH THE FOLLOWING THREE EXCEPTIONS:
1. #3 AND #4 COLUMN TIES AND BEAM STIRRUPS AND BREAKOUT DOWELS SHALL BE GRADE 40 WITH A MINIMUM TIED STRENGTH OF 40,000 psi.
2. ANY AND ALL REINFORCING TIES TO BE WELDED SHALL BE DEFORMED WELDABLE BAR (WB) THAT CONFORMS TO ASTM A706 GRADE 60.
3. UNLESS NOTED OTHERWISE (UNO) ON DRAWINGS.
4. WELDING OF REINFORCING SHALL BE WITH LOW HYDROGEN ELECTRODES IN CONFORMANCE WITH RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL AMERICAN WELDING SOCIETY, AWS-D1.4.
5. SPLICES OF REINFORCING BAR, IF REQUIRED, SHALL BE AVOIDED AT POINTS OF MAXIMUM STRESS. ALL SPLICES AND LAPS IN REINFORCING BARS SHALL CONFORM TO TYPICAL DETAIL B/SO.1. SPLICES SHALL BE MADE IN A REGION OF COMPRESSION, UNLESS SHOWN OTHERWISE.
6. REINFORCING BARS SHALL NEITHER BE WELDED NOR BENT BY HEATING, WHERE INSERTS REQUIRE WELDING TO PLATES, ANGLES OR THE LIKE, DEFORMED WELDABLE BARS SHALL BE USED.
7. ALL HOOKS IN REINFORCING BARS SHALL BE BENT 180 DEGREES WITH AN INSIDE DIAMETER OF 6 BAR DIAMETERS FOR BARS UP TO 1 IN. AND 8 BAR DIAMETERS FOR BARS OVER 1 IN. IN DIAMETER. EXTEND BARS A MINIMUM OF 4 BAR DIAMETERS BEYOND BEND. REFER TO STANDARD REBAR BEND DETAILS (A/SO.1) AND TYP. REBAR LAP LENGTH DETAILS (B/SO.1) FOR CLARIFICATION
8. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 WITH A YIELD STRENGTH OF 65000 psi. OR ASTM A497 WITH A TIED STRENGTH OF 70000 psi.
9. MINIMUM LAP OF WELDED WIRE FABRIC SHALL BE 6 INCHES OR ONE FULL MESH AND ONE HALF, WHICHEVER IS GREATER.
10. DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE, AND SPACING OR NUMBER AS THE VERTICAL REINFORCING, RESPECTIVELY. UNO.
- SPECIAL INSPECTION OF WELDS MADE TO STRUCTURAL STEEL (IE COLUMNS, BASE PLATES, SADDLES, BRACKETS, ETC.) SHALL BE MADE IN ACCORDANCE WITH AWS D1.1 AND SHALL BE MADE PRIOR TO THE COVERING OF SUCH WELDS.

INSPECTION OF STEEL CONSTRUCTION

- SPECIAL INSPECTION OF WELDS MADE TO STRUCTURAL STEEL (IE COLUMNS, BASE PLATES, SADDLES, BRACKETS, ETC.) SHALL BE MADE IN ACCORDANCE WITH AWS D1.1 AND SHALL BE MADE PRIOR TO THE COVERING OF SUCH WELDS.



BAR SIZE	D (BAR DIA.)	DIA.	A	B	HOOKS 90° BENDS
#3	3/8"	2 1/4"	2 1/2"	1 1/2"	4 1/2"
#4	1/2"	3"	2 1/2"	2"	4 1/2"
#5	5/8"	3 3/4"	2 1/2"	2 1/2"	7 1/2"
#6	3/4"	4 1/2"	3"	3"	9"

A STANDARD REBAR BENDS DETAIL

BAR SIZE	D (BAR DIA.)	LAP	HOOK EMBED
#3	3/8"	15"	6 1/2"
#4	1/2"	19 1/2"	8 1/2"
#5	5/8"	24"	10 1/2"
#6	3/4"	29"	12 1/2"

B TYP. REBAR LAP LENGTHS

DATE:	REVISION:
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**KC FAIRGROUNDS BUILDING
STRUCTURAL SPECIFICATIONS
KANE COUNTY, UTAH**
SCALE= NTS
REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

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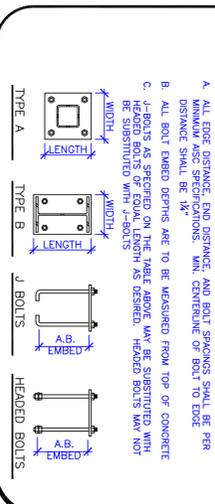
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- FOUNDATION PLAN NOTES:
1. TYP. MINIMUM EXTERIOR FOOTING EMBEDMENT SHALL BE 18" UNLESS NOTED OTHERWISE ON PLANS OR DETAILS
 2. REFER TO STRUCTURAL SPECIFICATIONS FOR ALL PAD GRADING AND PREPARATION AS WELL AS COMPACTION REQUIREMENTS
 3. TYP. SLAB SHALL BE 4" THICK AND REINFORCEMENT SHALL BE #3 BARS AT 24" o.c. EACH WAY CENTERED IN SLAB THICKNESS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS
 4. ALL CONCRETE EXCEPT RETAINING/FOUNDATION WALLS, FOOTINGS SUPPORTING RETAINING/FOUNDATION WALLS, AND PERISTALS SUPPORTING RETAINING/FOUNDATION WALLS, SHALL BE CAST WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. RETAINING/FOUNDATION WALLS, FOOTINGS SUPPORTING RETAINING/FOUNDATION WALLS, AND PERISTALS SUPPORTING METAL BUILDING COLUMNS SHALL HAVE A COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS.
 5. ALL INTERIOR SLABS ON GRADE TO BE CAST OVER MINIMUM 2" SAND OVER 10 MIL VISQUEEN OVER 6" TYPE II AGGREGATE BASE OR PROVIDE EQUIVALENT VAPOR/MOISTURE BARRIER. NOT RECD WHERE NO FLOOR COVERINGS TO BE APPLIED
 6. VERIFY ALL FOUNDATION AND SLAB DIMENSIONS W/ METAL BUILDING DRAWINGS. REFER TO METAL BUILDING DRAWINGS FOR BOLT DIAMETER AND LAYOUT FOR STEEL COLUMNS
 7. CONTRACTOR TO VERIFY LOCATION OF ALL ANCHOR BOLTS, HOLDOWN ANCHORS, INSERTS, PLUMBING, ETC., PRIOR TO PLACING CONCRETE DETAIL (A)
 8. REFER TO SHEET SO.1 FOR LAP SCHEDULE (B) AND STANDARD BENDS DETAIL (A)
 9. VERIFY ALL DIMENSIONS W/ ARCH'L DRAWINGS PRIOR TO PLACING CONCRETE
 10. CLEAR BUILDING PAD OF ALL LOOSE DEBR, SHRUBS, ORGANIC MATERIAL, ETC., PRIOR TO PLACEMENT OF CONCRETE

* FOUNDATION AND FOOTINGS SUPPORTING METAL BUILDING TO BE VERIFIED AFTER METAL BUILDING IS DESIGNED AND REACTIONS HAVE BEEN PROVIDED TO ENGINEER. ADDITIONAL FOOTINGS MAY BE REQUIRED TO ACCOMMODATE METAL BUILDING FRAMING.

FOOTING LABEL	FOOTING LENGTH/WIDTH	FOOTING THICKNESS	FOOTING EMBED	REQ'D REINFORCING	DETAIL REFERENCE
F1	4'-0" x 4'-0"	12"	PER DETAIL	(4) #5 BARS E.W. BOTTOM	---
F2	2'-4" x 2'-4"	12"	PER DETAIL	(3) #5 BARS E.W. BOTTOM	---
F3	5'-6" x 5'-6"	12"	PER DETAIL	(6) #5 BARS E.W. BOTTOM	P1
F4	3'-10" x 3'-10"	12"	PER DETAIL	(4) #5 BARS E.W. BOTTOM	P2
F5	REFER TO DETAIL 107	REFER TO DETAIL 107	REFER TO DETAIL 107	REFER TO DETAIL 107	P1
F6	REFER TO DETAIL 107	REFER TO DETAIL 107	REFER TO DETAIL 107	REFER TO DETAIL 107	P2

BASEPLATE SCHEDULE				
B.P. LABEL	B.P. TYPE	B.P. LENGTH/WIDTH	B.P. THICKNESS	ANCHOR BOLTS
B1	A	10' x 10'	3/8"	(4) #6 D.N. A.B. J BOLT
B2	B	OTHERS	OTHERS	(4) #6 D.N. A.B. HEADERS BOLT

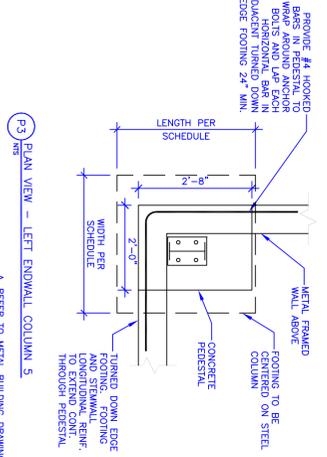
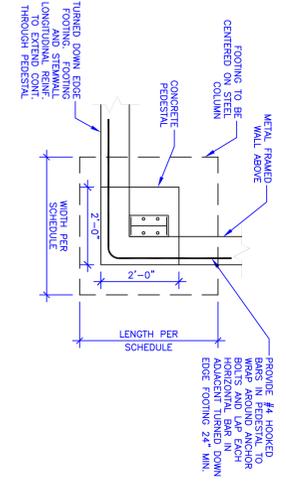
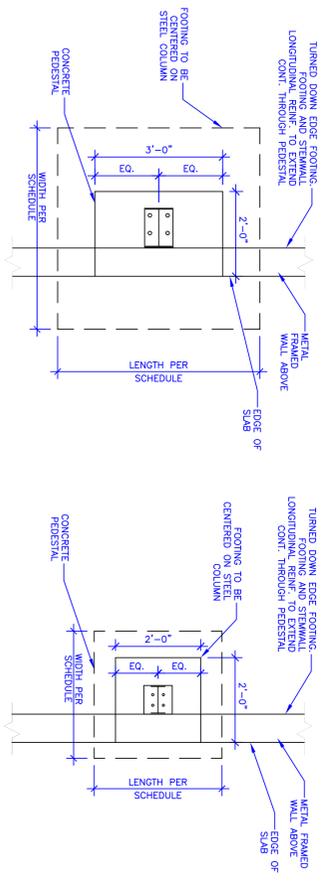
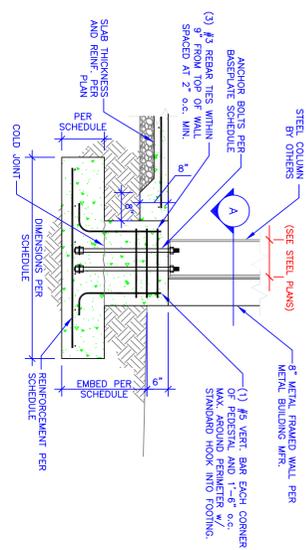


DATE:	REVISION:
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KC FAIRGROUNDS BUILDING
PLAN NOTES / SCHEDULES
KANE COUNTY, UTAH
 SCALE= NTS
 REDUCE SCALE BY 1/2 FOR 11x17 DRAWINGS

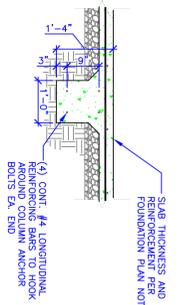
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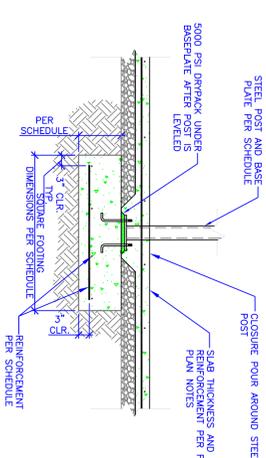


- A. REFER TO METAL BUILDING DRAWINGS FOR EXACT LOCATION AND PLACEMENT OF STEEL COLUMNS AND ANCHOR BOLTS.
- B. REFER TO FOOTING SCHEDULE FOR FOOTING SIZE AND CONFIGURATION.
- C. PROVIDE ADEQUATE CONCRETE CLEAR COVER AROUND ANCHOR BOLTS. TYP.

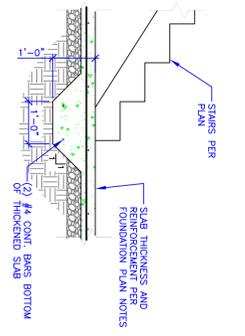
108 TYP. FOOTING AT METAL BUILDING COLUMN
N.T.S.



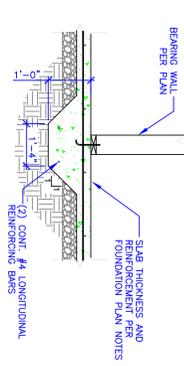
106 CONT. TIE GRADE BEAM
N.T.S.



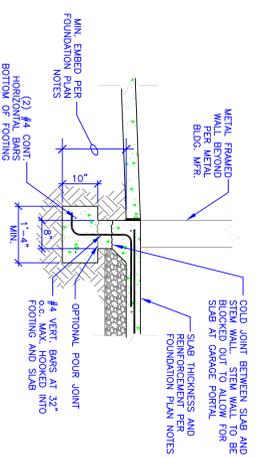
105 STEEL POST AT FOOTING
N.T.S.



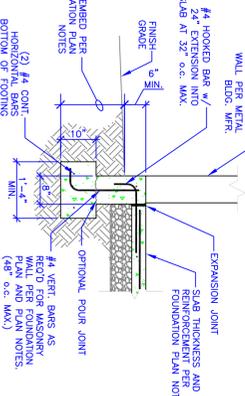
104 TYP. THICKENED SLAB FOOTING AT STAIRS
N.T.S.



103 CONT. INTERIOR THICKENED SLAB FOOTING
N.T.S.

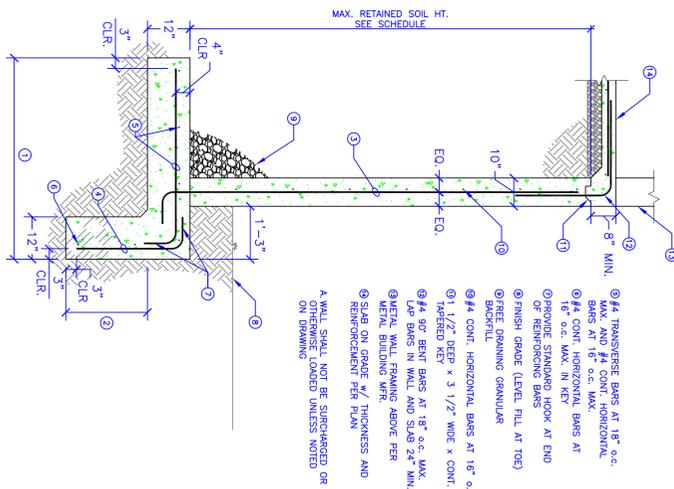


102 TYP. FOOTING AT GARAGE PORTAL
N.T.S.



101 TYP. EXTERIOR PERIMETER FOOTING
N.T.S.

RETAINED SOIL FOOT. HEIGHT	KEY	FOOT. WIDTH	FOOT. DEPTH	WALL VERT. REINF.	KEY
5'-0"	1	2'-9"	0'-8"	#4 @ 18" o.c.	1
7'-0"	2	3'-6"	1'-8"	#5 @ 14" o.c.	2
9'-6"	3	4'-9"	1'-11"	#5 @ 9" o.c.	3



107 CONCRETE FOUNDATION/RETAINING WALL
N.T.S.

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